

MONASH PIPE EVALUATION PLATFORM





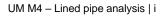
Australian Government Department of Industry, Innovation and Science Business Cooperative Research Centres Program



MONASH Civil Engineering University



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## **QUALITY INFORMATION**

Document: User Manual, Module 4 – Lined pipe analysis Edition date: 18-10-2021 Edition number: 1.3 Prepared by: Benjamin Shannon Reviewed by: Rukshan Azoor, Guoyang Fu

## **Revision history**

Revision	Revision date	Details	Revised by
1.1	05-07-21	Updated	Ben Shannon
1.2	27-08-21	Added descriptions of each property input	Ben Shannon
1.3	18-10-21	Final revision	Ben Shannon





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## ACKNOWLEDGEMENTS

Nu Flow Technologies

The Australian Government, through the Cooperative Research Centre, provided funding for the Smart Linings for Pipe and Infrastructure Project that produced this report. The CRC Program supports industry-led collaborations between industry, researchers and the community.



Australian Government

Department of Industry,

Innovation and Science

**Business** Cooperative Research

**Centres Program** The project was led by the Water Services Association of Australia (WSAA) and included the following project partners, all of whom contributed expertise, labour, funding, products or trial sites to assist in the delivery of this project.

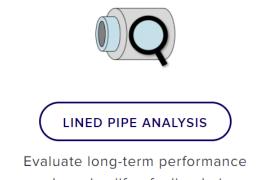
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# **INTRODUCTION**

This document outlines the methods and processes used in the lined pipe analysis (Figure 1), and provides guidelines for its use.



and service life of a lined pipe Figure 1. Lined pipe analysis module icon in the user interface

The primary function of the lined pipe analysis module is to provide an estimate of the liner service life or thickness and a net present value cost analysis. The long-term analysis approach or NPV cost analysis options are provided at the start of the module (Figure 2).

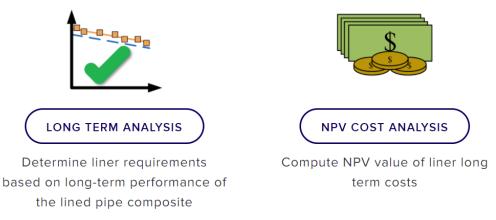


Figure 2. Long-term analysis and NPV cost analysis

## 1 LONG-TERM ANALYSIS

The long-term analysis can be conducted for either a single pipe or multiple pipe analysis<sup>1</sup>. The calculation process is organised into a four-step simple workflow, with basic and advanced properties, to determine either:

- Liner design thickness or
- Estimate service life.

The thickness or service life calculations are based on the formulas developed at Monash University and from standards (ASTM F1216 2016; AWWA 2019).

<sup>&</sup>lt;sup>1</sup> Multiple pipe analysis has not been implemented at this stage.





### 1.1 Step I-Liner properties

In the liner properties step, the user is required to select the type of liner, either **Spray or CIPP**, the **liner class** (A, B, C, or D), and the **Reinforcement type** (for CIPP only, categorised as either GFRP (Glass fibres) or FRP (Polymeric fibres)). The user interface is given below in Figure 3. Two different approaches can be selected. Liner thickness design will determine the minimum wall thickness required for the lined pipe to survive the inputted liner design service life, whereas Estimate service life uses a liner thickness value to determine the service life of the lined pipe.

......

	SINGLE PIPE	ANALYSIS	
LINER PROPERTIES HOST P	IPE PROPERTIES/LOADING	COMPOSITE CHECK	OUTPUTS
	Step I: Liner Pr	operties	
Locate by Pipe IE	):	Q	
Liner type: Reinforcement type : Liner class:	Spray V ? Se	lect Approach O Liner Thickness Design Liner design service life (years): t O Estimate Service Life Liner thickness (mm): T <sub>L</sub>	50 <b>?</b> 3 <b>?</b>
Advanced Parameters (for the Liner)			Click to show
	Next 🕨		

Figure 3. User interface for the failure history step in the pipe level analysis of the liner selection module

- Liner thickness  $(T_L)$  The liner thickness required for the specified liner service life. Units are in mm. Liner thickness can be noted as just the reinforcing layer thickness  $(T_r)$  (for CIPP only) or total liner thickness  $(T_L)$ , however this must be noted by the applicator to avoid interchanging of terms.  $0 \le T_L \le 100 \ mm$ .
- Liner design service life (*t*) The platform will calculate the minimum design liner thickness ( $T_L$ ) based on the service life (time in years) examined. Units are in years.  $0 \le t \le 100$  years.

Advanced parameters are based on generic liner properties, which are explained in the theory manual (TM M4 Part 1 – Determination of liner long-term properties, TM Part 2 – Lined pipe analysis and product standards available from WSAA), were found from literature and Monash University testing (Figure 4).

Generic liner parameters were developed from the testing regime conducted at Monash University and are shown below.

Table 1 shows the short-term generic liner properties used in the Platform. These properties were found from tensile testing, flexural testing and thermal expansion testing. Table 2 shows the long-term generic liner properties from creep rupture/hydrostatic design basis testing and creep. Table 3 shows the fatigue properties from fatigue testing used in the Platform. For further details on how to use the long-term deterioration coefficients (Table 2 and Table 3) please see TM M4 Part 1 – Determination of liner long-term properties.





		Ten strength			modulus Pa)	Flexu		Flex modulus		Poisson's ratio	Thermal coefficient (mm/mm/°C) <sup>2</sup>
Liner	Class	$\sigma_{th}$	$\sigma_{ta}$	$E_{th}$	$E_{ta}$	$\sigma_{fh}$	$\sigma_{fa}$	$E_{fh}$	E <sub>fa</sub>	$ u_L $	α
CIPP	C <sup>3</sup>	45	30	0.4	0.07	0	0	0	0	0.5	0.00005
CIPP	в	75	50	3	2	40	40	2	2	0.31	0.00005
CIPP	A	75	65	6	5	60	60	6	5	0.31	0.00005
Spray	A-D	35	35	3	3	60	60	3	3	0.35	0.00005

### Table 1. Generic short-term liner properties for liner type and class

Table 2. Generic long-term liner properties for liner type and class

		Strength reconficient		Coefficients modulus redu		Coefficient modulus red	
Liner	Class	$x_l$	cl	$x_{lc}$	C <sub>lc</sub>	x <sub>lc</sub>	C <sub>lc</sub>
CIPP	С	-0.017	0.78	1	-0.106	0.88	-0.14
CIPP	В	-0.017	0.78	1.5	-0.17	1.1	-0.12
CIPP	A	-0.017	0.78	1.5	-0.17	1.1	-0.12
Spray	A-D	-0.03	0.65	2.16	-0.278	2.16	-0.278

### Table 3. Generic fatigue long-term liner properties for liner type

	Coefficients for f reduction	
Liner type/reinforcement type	$x_f$	C <sub>f</sub>
Spray	-0.0368	1.023
GFRP	-0.0493	1.146
FRP	-0.0133	0.946

Figure 4 shows the user interface for advanced liner properties. The advanced parameters are gathered from the generic liner properties (Table 1, Table 2, and Table 3). Users can modify these advanced parameters based on their preferred liner.

<sup>&</sup>lt;sup>2</sup> Note: Coefficient of thermal expansion values have been approximated from literature. Property testing still needs to be conducted. <sup>3</sup> Flexural strength and modulus of Class C CIPP liner were based on a flexible liner (highly ductile resin), hence no flexural strength was recorded.



Click to hide



-0.017

Values

0.78

#### ✓ Advanced Parameters (for the Liner)

Liner Properties: Short-term tensile strength (hoop) (MPa): 75 ? Coefficient of thermal expansion (mm/mm/°C): a 0.00005 ? Short-term tensile strength (axial) (MPa): Short-term flexural modulus (hoop) (GPa): En  $\sigma_{ts}$ 65 6 ? Short-term tensile modulus (hoop) (GPa): Short-term flexural modulus (axial) (GPa): E+ En 6 5 ? Short-term tensile modulus (axial) (GPa): Adhesion strength of the liner to host pipe substrate Eta 5  $\sigma_{ac}$ 0.25 (MPa) ? Poisson's ratio of liner:  $v_L$ Factor of Safety for imperfections: Ν ? 0.31 2 Wet strength reduction factor: ?  $\varphi$ Cyclic surge or pressure transient properties: Number of recurring cyclic surge pressure cycles per ? Pressure transient factor n<sub>PC</sub> 2 2 Primary pressure wave stress / secondary pressure wave stress: ? Total number of surge pressure cycles for service life of pipe/lined pipe: n<sub>TP</sub>¢ 365000 1  $\Delta \sigma_1$ Reduction curves of liner long-term properties as a function of time due to deterioration: Type of deterioration Strength reduction Creep modulus reduction Fatique strength reduction Orientations Axial Hoop Hoop Hoop Deteoriation Coefficients XI CI Clc XIc Clc CIF XIC XIf

Figure 4. Advanced parameter for liner selection. Values are prefilled based on the basic mode options selected by the

-0.17

1.1

-0.12

-0.0133

0.927

1.5

. user.

- Short-term tensile strength of the liner ( $\sigma_t$ ,  $\sigma_{th}$  and  $\sigma_{ta}$ ) Short-term (initial) tensile strength of the liner (from wet testing testing conducted with saturated specimens) is the ultimate tensile strength of the liner. Testing is conducted in both axial and hoop directions (if liner is bi-directional, where subscript *h* is in the hoop direction and subscript *a* is in the axial direction). Testing is conducted from tensile tests (AS 1145 2001, ASTM D638 2014, ASTM D3039 2017) and units are in MPa.  $5 \le \sigma_t \le 500 MPa$ .
- Short-term tensile modulus of elasticity of the liner ( $E_L$ ,  $E_t$ ,  $E_{th}$  and  $E_{ta}$ ) Short-term (initial) tensile modulus of elasticity or Young's modulus of the liner (from wet testing testing conducted with saturated specimens) is the stress divided by strain of the linear proportion of the stress strain curve. Testing is conducted in both axial and hoop directions (if liner is bidirectional, where subscript *h* is in the hoop direction and subscript *a* is in the axial direction). Testing is conducted from tensile tests (AS 1145 2001, ASTM D638 2014, ASTM D3039 2017) and units are in GPa.  $0.01 \le E_L \le 100$  GPa.
- Poisson's ratio of the liner (ν<sub>L</sub>) Poisson's ratio is the measurement of deformation in pipe liner material in a direction perpendicular to the direction of the applied force. Units are dimensionless.
   0.1 ≤ ν<sub>L</sub> ≤ 0.5.
- Coefficient of thermal expansion (α) Coefficient of thermal expansion or contraction is the change in size with change in temperature. The units are in mm/mm/°C. 0 ≤ α ≤ 0.01 mm/mm/°C.
- Short-term flexural modulus of elasticity of the liner ( $E_{fh}$  and  $E_{fa}$ ) Short-term (initial) tensile modulus of elasticity or Young's modulus of the liner (from wet testing testing conducted with saturated specimens) is the stress divided by strain of the linear proportion of the stress strain curve.





Testing is conducted in both axial and hoop directions (if liner is bidirectional, where subscript *h* is in the hoop direction and subscript *a* is in the axial direction). Testing is conducted from flexural tests (ISO 14125 (1998), (AS 1145 2001, ASTM D790 2017) and units are in GPa.  $0.01 \le E_f \le 100 \text{ GPa}$ .

- Adhesion strength of the liner to host pipe substrate ( $\sigma_{ad}$ ) Adhesion strength of the liner to host pipe substrate. Adhesion tested on a variety of surfaces, including CML (if CML lined pipe) shall be determined using any or all of the following standards: ASTM D4541 for metal, ASTM D7234 for AC and CML, or Pull off adhesion testing to AS 3894.9 for all CIPP classes that require adhesion for sealing (e.g. at ends or service connections) or bonding to the host pipe (Class C). Units are in MPa.  $0 \le \sigma_{ad} \le 100 MPa$ .
- Factor of safety for liner imperfections (N<sub>i</sub>) Factor of safety for liner imperfections (variable wall thickness in polymeric spray lining and folds in CIPP lining), depends on the imperfection type. The following factors of safety for liner imperfections were selected:
  - Uneven thickness (polymeric spray),  $N_i = 1.5$
  - Folds (CIPP),  $N_i = 2$
- Wet strength reduction factor ( $\phi_s$ ) If the liner is subjected to plasticisation due to water saturation a wet strength reduction factor must be applied to reduce short-term and long-term strength properties. Ideally, testing should be conducted in wet conditions if liner plasticises when saturated. This is more common in polymeric spray liners than CIPP liners. The wet factor reduction ranges from 0.5–1, however the default value is set as 1 in the Monash Pipe Evaluation Platform as the tests are assumed to be conducted in water.

For example:

$$\phi_s = \frac{\sigma_{t,wet}}{\sigma_{t,dry}}$$

where  $\sigma_{t,wet}$  is the wet tensile strength of the liner (MPa) and  $\sigma_{t,dry}$  is the dry tensile strength of the liner (MPa). These properties can be interchanged with  $\sigma_{th}$  and  $\sigma_{ta}$  if property tests were conducted in water. The use of tensile/flexural and hoop/axial shall be consistent for both wet and dry properties to determine strength reduction factors.  $0.5 \le \phi_s \le 1$ .

- Number if recurring cyclic surge pressure cycles (cycles/day)  $(n_{PC})$  Number of pressure transients per day are used to determine if the host pipe or liner will fail from fatigue. Consistent pressure transients may reduce the life of the host pipe and liner. The default value is set at 2 per day, for pump start-up and pump shutdown.  $0 \le n_{PC} \le 100$  cycles per day.
- Primary pressure wave stress / secondary pressure wave stress (Δσ<sub>0</sub>/Δσ<sub>1</sub>) The magnitude of the initial pressure cycle wave stress (Δσ<sub>0</sub>) divided by the magnitude of the secondary pressure cycle wave stress (Δσ<sub>1</sub>). See Figure 5 for further details. A value of 1 indicates the second pressure wave is as large as the first and is conservative. 0 ≤ Δσ<sub>0</sub>/Δσ<sub>1</sub> ≤ 5.

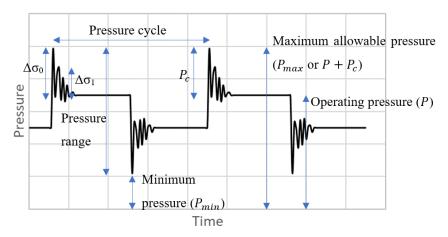


Figure 5. Pressure information used in the MPEP.





• **Cyclic surge factor**  $(n_f)$  - A surge factor  $(n_f)$  or pressure transient factor is used to incorporate the effect of secondary cycles from cyclic surge pressure. A simplistic approach is to use a pressure cycle is equal to 2 primary cycles due to the decaying surge. Alternatively, the following equations can be used to calculate a factor for number of pressure transients (Joseph 1979, PIPA 2018).

$$n_f = 1 + \frac{1}{\left(\frac{\Delta\sigma_0}{\Delta\sigma_1}\right)^{3.2}}$$

where  $n_f$  is the cyclic surge factor (from 1 to 2),  $\Delta \sigma_0$  is the magnitude of the initial pressure cycle wave and  $\Delta \sigma_1$  is the magnitude of the secondary pressure cycle.

• Total number of pressure transients for the life of the pipe/lined pipe  $(n_{TPC})$  - The total number of pressure transients or cyclic pressure surges can be calculated based on the following equation

$$n_{TPC} = n_{PC} \times n_f \times t \times 365$$

where  $n_f$  is the cyclic surge factor (from 1 to 2) and *t* is the time in years, where in this case is the liner design service life in years.  $0 \le n_{TPC} \le 10^7$ .

• **Tensile rupture strength of the liner** ( $\sigma_{tl,r}$ ,  $\sigma_{thl,r}$  and  $\sigma_{tal,r}$ ) - Tensile rupture strength or long-term tensile strength (hoop or axial) of the liner at *x* years is based on creep rupture (ASTM D2990 2001) or hydrostatic design basis (ASTM D2992 2018) for a specified design life (years). The normalised curve can be multiplied by the initial tensile strength in MPa. Wet conditions should be used.

The following equation is used to approximate the deterioration curve based on hydrostatic design basis or creep rupture testing. The equation uses the 97.5% lower prediction limit results, to account for variability in the results and is simplified to a logarithmic curve. Alternatively, the hydrostatic design stress value can be used if the service life is 50 years.

$$\sigma_{tl,r} = \sigma_t(x_l ln(t_h) + c_l)$$

where,  $\sigma_{tl,r}$  is the tensile rupture strength at a particular service life ( $\sigma_{tl}$  can be substituted for  $\sigma_{thl,r}$  and  $\sigma_{tal,r}$  for the hoop and axial values respectively),  $\sigma_t$  is the short-term tensile strength ( $\sigma_t$  can be substituted for  $\sigma_{th}$  and  $\sigma_{ta}$  for the hoop and axial values respectively),  $x_l$  and  $c_l$  are creep rupture deterioration constants and  $t_h$  is the time in hours.

Note: The same formula can be applied to approximate the long-term flexural strength by substituting short-term wet flexural strength values, however flexural creep rupture testing is preferred.

- **Coefficient for strength reduction** (*x*<sub>*l*</sub>) Deterioration coefficient for creep rupture or hydrostatic design basis normalised stress vs. time (hours) curve. Will be a negative value to correspond with rupture strength of the liner.
- **Coefficient for strength reduction** (*c*<sub>*l*</sub>) Deterioration coefficient for creep rupture or hydrostatic design basis normalised stress vs. time (hours) curve. Will be a positive number.
- **Tensile creep modulus of the liner** ( $E_{tl}$ ,  $E_{thl}$  and  $E_{tal}$ ) Creep modulus (hoop or axial) is defined as the ratio of applied stress to the time-dependent strain and is determined based on creep testing (ASTM D2990 2001) or hydrostatic strain basis (ASTM D2992 2018) for a specified design life (years). Wet conditions should be used.

The tensile creep modulus  $(E_{tl})$  was estimated using Findley's power law from creep tests up to 20,000 hours and extrapolated up to 100 years. The creep modulus in tensile and axial can be found by multiplying the corresponding creep retention factor (*CRF*) by the initial or short-term modulus.

$$E_{tl} = CRF \cdot E_t$$

where  $E_{tl}$  is the tensile creep modulus and  $E_t$  is the short-term tensile modulus. The following equation can be used to determine the CRF and subsequently the long-term tensile modulus at any point in time.

$$CRF = x_{lc}(t_h)^{c_{ld}}$$

where  $x_{lc}$  and  $c_{lc}$  are factors to determine creep modulus factor based on a power law decay, and  $t_h$  is the time in hours.





Note: The same formula can be applied to approximate the flexural creep modulus by substituting short-term wet flexural strength values, however flexural creep testing is preferred. This formula should not be used for less than 100 hours.

• **Creep retention factor (***CRF***)** - The creep retention factor (*CRF*) is the creep modulus divided by the initial or short-term modulus of the liner (in either hoop or axial directions).

$$CRF = \frac{E_{tl}}{E_t}$$

- **Deterioration coefficient for creep**  $(x_{lc})$  Deterioration coefficient for creep modulus vs. time (hours) curve based on testing results and Findley's power law. Will be a positive number.
- **Deterioration coefficient for creep (***c<sub><i>lc*</sub>**)** Deterioration coefficient for creep modulus vs. time (hours) curve based on testing results and Findley's power law. Will be a negative number.
- Fatigue strength (hoop) of the liner ( $\sigma_{thl,f}$ ) Fatigue strength or long-term tensile fatigue deterioration strength (hoop) at *x* years is based on fatigue curves based on ISO 13003 (2003) or cyclic pressure design basis (ASTM D2992 2018) for a specified design life (years). The normalised stress vs. number of cycles to failure ( $N_f$ ) curve can be multiplied by the initial tensile strength in MPa. Wet conditions should be used.

The following equation is used to approximate the deterioration curve based on cyclic pressure design basis or fatigue cyclic testing. The equation uses the 95% lower prediction limit results, to account for variability in the results and is simplified to a logarithmic curve. Alternatively, the hydrostatic fatigue stress value can be used if the service life is 50 years.

$$\sigma_{thl,f} = \sigma_{th} \big( x_{lf} ln(n_{TPC}) + c_{lf} \big)$$

where,  $\sigma_{thl,f}$  is the long-term fatigue strength at a particular service life,  $\sigma_{th}$  is the (wet) short-term tensile strength,  $x_{lf}$  and  $c_{lf}$  are fatigue deterioration constants and  $n_{TPC}$  is the total number of surge pressure cycles for life of pipe/lined pipe.

- Coefficient for fatigue strength reduction  $(x_{lf})$  Deterioration coefficient for fatigue or cyclic pressure design basis normalised stress vs. time (hours) curve. Will be a negative value to correspond with deteriorated strength of the liner.
- Coefficient for fatigue strength reduction  $(c_{lf})$  Deterioration coefficient for fatigue or cyclic pressure design basis normalised stress vs. time (hours) curve. Will be a positive number.

### 1.2 Step II – Host pipe properties/soil properties/loading

Implementing the host pipe properties are similar to the other modules (Module 2 and 3, pipe failure analysis and liner selection respectively), where host pipe properties are gathered from pipe cohorts (see TM M2 Part 1 - Pipe and soil cohorts). The properties are prefilled based on the pipe material, pipe segment installation year, nominal diameter and operating pressure. The properties are shown in Figure 6.





1 LINER PROPERTIES	2 HOST PIPE PROPERTIES/LOADING	COMPOSITE CHECK	OUTPUTS	
	Step II: Host pipe propertie	s/Soll propertles/Loading		
Pipe mater	al: CICL V?	Cement mortar liner thickness:	Light V ?	
Pipe segment installation yea	r: 1950 <b>?</b>	Liner external diameter (mm):	D <sub>L</sub> 146.40 ?	
Nominal diameter (mm	): DN 150 ?	Burial depth (mm):	H 1000 ?	
Pipe internal diameter (mm	): D 152.4 ?	Safety factor for host pipe:	N 1 ?	
Pressure head (m	): h 75 ?	Soil type: Sand	<	
> Advanced Parameters (for the	e Host Pipe)			Click to show
	<b>▲</b> Back	Next 🕨		

Figure 6. User inputs host pipe properties/Soil properties/loading

The host pipe will be assigned a standard based on the pipe segment installation year, and an internal diameter based on the pressure head (operating pressure). Users can increase the pressure head value if the pipe class is higher (larger wall thickness). A warning will appear if the pipe properties are not found in the cohort (Figure 7). This can arise if the nominal diameter is not close to the ones found in standards or operating pressure is high.

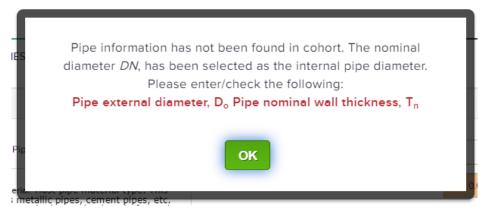


Figure 7. Warning when cohort information cannot be found.

- **Pipe material** Host pipe material type. This includes metallic pipes, cement pipes, plastic pipe, etc. In the Monash Pipe Evaluation Platform, the following pipe materials are implemented: Cast Iron (CI, CICL), Asbestos cement (AC), Mild Steel (MS, MSCL) and Ductile Iron (DICL).
- **Pipe segment installation year** Construction year of the pipe, burial year of the pipe or pipe installation year. 1800 to 2021.
- **Pipe nominal diameter** (*DN*) Nominal diameter of the pipe or internal diameter of the pipe. Typically, the nominal diameter is expressed in mm conveniently rounded to roughly the manufactured internal diameter, however the imperial terms use inches. A DN150 pipe has an internal diameter of 150 mm (Imperial, DN6 is 6 inch). Note that in some pipes, such as AC the pipe outer diameter (*OD* or *D*<sub>0</sub>) remains constant to allow for fitting compatibility, inner diameter reduces with an increase in pressure Class (wall thickness). Nominal diameter can be used as an approximate for internal diameter (*D*) if





internal diameter is unknown. Otherwise, cohort values can be used to determine the host pipe internal and external diameters.  $50 \le DN \le 3000$  mm.

• **Pipe internal diameter (***D***)** - Pipe internal diameter is the internal diameter of the host pipe, not including liners (e.g. CML). Internal diameter is found from pipe cohorts and standards, however the pipe nominal diameter (DN) can also be used as an approximation if internal diameter is unknown. The internal diameter units are expressed in mm. Figure 8 shows the different terms used in pipe.

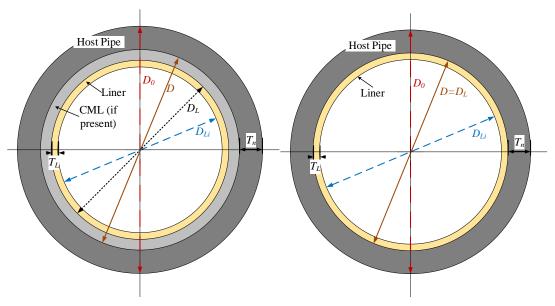


Figure 8. Labelling of pipe and liner term, left with CML, right without CML

- **Pressure head (***h***)** The operating pressure converted to a meter head value. Used to determine the pressure class of the pipe. Units are in m.  $0 \le h \le 300$  m.
- **Cement mortar liner thickness** Cement mortar liner (CML) thickness, which can be either *in situ* or factory lined. Options include Light, Medium and Heavy. For use with CL pipes only.
- Liner external diameter  $(D_L)$  The liner external diameter in mm. The liner external diameter can be equal to the pipe internal diameter (D) if no CML is present. If a CML is present, D and  $D_L$  need to be used for calculation. See Figure 8.  $0 \le D_L \le 5000$  mm.
- Burial depth (*H*) The depth of the pipe from the ground surface level to the crown of the pipe Units are in mm.  $0 \le H \le 10000$  mm.
- Safety factor for host pipe (N) A safety factor for host pipe. A value of 1 indicates no factor of safety. 1≤ N ≤ 5.
- Soil type Soil type could be any of the following soils: sand, loamy sand, sandy loam, fine sandy loam, loam, silty loam, sandy clay loam, fine sandy clay loam, clay loam, silty clay loam, sandy clay, light clay, silty clay, medium clay, heavy clay. The soil type names are from AS 4419 (2018). The users can select a soil type from the list and the soil properties will be prefilled.

If further information is required to be changed by the user, the Advanced parameters tab can be selected (Figure 9). The host pipe properties including material properties and sizes (gathered from cohort data – See Theory manual Pipe and soil cohorts) can be modified as well as soil properties (gathered from soil cohort data). The soil type dropdown box will automatically change the soil properties values in the advanced parameters tab.





Host Pipe:		AC Pipes	
Pipe external diameter (mm):	D <sub>o</sub> 177.20 ?	Internal deterioration rate for AC Pipes:	C <sub>ACi</sub> 0.123 ?
Pipe Class:	C ?	External deterioration rate for AC Pipes:	<i>c<sub>ACe</sub></i> 0.1114 ?
Standard:	BS 78 (1938)	Steel/Metallic Pipes	
Pipe nominal wall thickness (mm):	T <sub>n</sub> 12.4 ?	Lateral extension rate (mm/y):	r <sub>sh</sub> 0.00 ?
Estimated external uniform corrosion (mm):	<i>T<sub>ext</sub></i> ?	Long-term, steady state corrosion rate of metallic pipes (mm/y):	r <sub>s</sub> 0.0042 ?
Estimated internal uniform corrosion (mm):	T <sub>ext</sub>	Intercept parameter for long-term corrosion of metallic pipes:	c <sub>s</sub> 1.95 ?
Pipe wall thickness allowing for uniform corrosion (mm):	7 12.40 ?	Transition period between short-term and long-term corrosion (y):	τ 15 ?
Ultimate tensile strength of the host pipe (MPa):	σ <sub>t</sub> 124 ?		
Yield strength of the host pipe (MPa):	σ <sub>y</sub> 124 ?		
Pipe elastic modulus (GPa):	E <sub>ta</sub> 110		
Poisson's ratio of host pipe:	v <sub>p</sub> 0.25 ?		
Host pipe fracture toughness (MPa $\cdot$ m $^{1/2}$ ):	K <sub>IC</sub> 14.5		
Loading Conditions:			
Operating pressure (MPa):	P 0.7 ?	Rotation angle ("):	θ 1 ?
Recurring cyclic surge pressure (MPa):	P <sub>c</sub> 0.3 ?	Temperature change (°C):	Δ7 5 ?
Vacuum pressure (MPa):	P <sub>v</sub> 0.1 ?	Live load (pressure) at the burial depth (MPa):	wq 0.03 ?
Height of water above pipe (mm):	H <sub>w</sub> 500 ?		

Figure 9. Advanced properties for host pipe and soil

- Pipe external diameter (D<sub>0</sub>) External diameter of the host pipe, also known as pipe outside diameter (OD), not including external coatings (e.g. bitumen). Pipe external diameter can be found from pipe cohorts and standards. The external diameter units are expressed in mm. See Figure 8. 50 ≤ D<sub>0</sub> ≤ 5000 mm.
- **Pipe pressure class** Pipe pressure class designed to withstand a certain pressure over a certain number of years. A safety factor would be included. Pipe pressure class can be used to gather pipe wall thickness, which are used in determining remaining life of the host pipe. Pipes with a higher-pressure class, have a larger wall thickness to withstand higher internal pressures. Typically, pressure classes are expressed as A to D for cast iron pipes and A to F for Asbestos cement pipes.
- Standard Standard that pipe cohort values have been used from.
- Pipe nominal wall thickness () The original wall thickness of the pipe (Figure 4). Units are in mm.
   T<sub>n</sub> > 0.
- **Pipe nominal wall thickness**  $(T_n)$  The original wall thickness of the pipe (Figure 10). Units are in mm.  $T_n > 0$ .
- Estimated external uniform corrosion  $(T_{ext})$  the pipe wall thickness reduction caused by external corrosion (Figure 10). Units are in mm.  $0 < T_{ext} < T_n$ .
- Estimated internal uniform corrosion  $(T_{int})$  the pipe wall thickness reduction caused by internal corrosion (Figure 10). Units are in mm.  $0 < T_{int} < T_n T_{ext}$ .





• Pipe wall thickness allowing for uniform corrosion (*T*) - the actual pipe wall thickness after accounting for external and internal corrosion (Figure 10). Units are in mm. Note that  $T = T_n - T_{ext} - T_{int}$ .

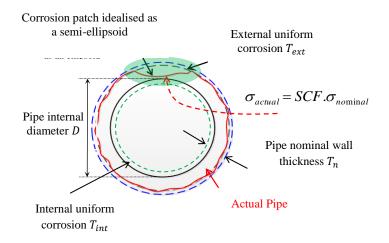


Figure 10. Cross section of a corroded pipe

- Ultimate tensile strength  $\sigma_t$ : the maximum stress that a material can withstand while being stretched or pulled before breaking. Units are in MPa.  $0 < \sigma_t \le 1000 MPa$ .
- Yield strength σ<sub>y</sub>: also known as specified minimum yield strength (*SMYS*) is the stress corresponding to the yield point at which the material begins to deform plastically. Units are in MPa. 0 < σ<sub>y</sub> ≤ 1000 MPa.
- Pipe elastic modulus *E<sub>p</sub>*: also known as Young's modulus or modulus of elasticity, it is the slope of stress–strain curve in the elastic deformation region for the pipe material. Units are in GPa. 1 ≤ *E<sub>p</sub>* ≤ 150 GPa.
- Poisson's ratio ν<sub>p</sub>: the measurement of deformation in pipe material in a direction perpendicular to the direction of the applied force. Units are dimensionless. 0.1 ≤ ν<sub>p</sub> ≤ 0.4.
- **Fracture toughness**  $K_{IC}$ : the resistance of materials to the propagation of cracks under an applied stress. Units are in MPa $\sqrt{m}$ .  $0 < K_{IC} \le 100 \text{ MPa}\sqrt{m}$ .
- Internal deterioration rate for AC pipes (c<sub>ACi</sub>) Internal deterioration rate for AC pipes. Rates can be calculated from phenolphthalein testing or approximated based on utility data. Units are in mm/y. 0 ≤ c<sub>ACi</sub> ≤ 0.5 mm/y.
- External deterioration rate for AC pipes (c<sub>ACe</sub>) External deterioration rate for AC pipes. Rates can be calculated from phenolphthalein testing or approximated based on utility data. Units are in mm/y. 0 ≤ c<sub>ACe</sub> ≤ 0.5 mm/y.
- Lateral extension rate  $r_{sh}$ : the increment of corrosion patch length and width per year. Half of the corrosion patch length  $(a_t)$  and half of the corrosion patch width  $(b_t)$  after t years can be calculated by  $a_t = a_0 + r_{sh} \times t$ ,  $b_t = b_0 + r_{sh} \times t$  respectively, where  $a_0$  and  $b_0$  are the current half of the patch length and half of the patch width respectively. Units are in mm/y.  $0 \le r_{sh}$ .
- Long-term corrosion rate  $r_s$ : the long-term steady state corrosion rate of metallic pipes (mm/year) in the exponential corrosion model (Figure 11).





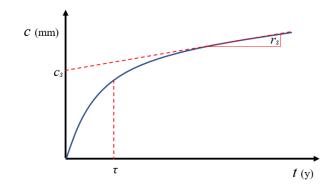


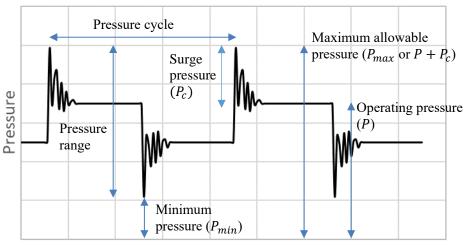
Figure 11. Exponential corrosion model

- Intercept parameter for long-term corrosion  $c_s$ : the intercept parameter between initial and long-term corrosion rate of metallic pipes (mm) in the power law corrosion model (Figure 11).
- Transition period between short-term and long-term corrosion  $\tau$ : the time it takes for the pit depth to attain around 63% of the maximum value in the absence of long-term corrosion rates (i.e.,  $r_s = 0$ ). If no value is available, then a value of 15 years is suggested to be used according to a careful review of the existing data in literature (Figure 11).
- Soil modulus *E<sub>s</sub>*: an elastic soil parameter used in the settlement, compression or movement of soils. It is the slope of stress–strain curve in the elastic deformation region for the soil. Units are in MPa.
   0 < *E<sub>s</sub>* ≤ 300 *MPa*.
- Lateral earth pressure coefficient: the lateral earth pressure coefficient at rest and can be calculated by:

$$k = 1 - \sin \emptyset \tag{1}$$

where  $\emptyset$  is the soil friction angle.  $0 \le k \le 1$ .

- Soil unit weight γ<sub>s</sub>: the ratio of the total weight of soil to the total volume of soil. Soil unit weight or bulk unit weight is the unit weight of soil and varies for different soil types. Units are in kN/m<sup>3</sup>. The values are typically between 15 kN/m<sup>3</sup> to 20 kN/m<sup>3</sup>. 0 < γ<sub>s</sub> ≤ 30 kN/m<sup>3</sup>.
- **Operating pressure** *P*: the operating pressure or maximum work/working pressure is the maximum internal pressure sustained that the liner is anticipated to be subjected to, including diurnal pressure fluctuations but not including recurring cyclic surge pressure (Figure 12). Units are in MPa.  $0 \le P \le 3$  MPa.







### Figure 12. Types of pressures

- **Recurring cyclic surge pressure**  $(P_c)$  Frequent repetitive pressure variations or cyclic surge pressures (water hammer) that are common in pressure pipe network systems (pump start-up or shutdown and normal valve opening and closings) (Figure 12). Units are in MPa.  $0 \le P_c \le 1$  MPa.
- Vacuum pressure  $(P_v)$  A negative pressure that a pipe may experience when subjected to a sudden displacement in water, such as dewatering, which results in a vacuum pressure. Vacuum pressure ranges from 0 MPa (no vacuum) to 0.1 MPa (high vacuum). If pipe is subjected to vacuum loading a suggested value of 0.1 MPa is used in the Monash Pipe Evaluation Platform. Units are in MPa.  $0 \le P_v \le 0.1$  MPa.
- Height of water above pipe, measured from pipe crown  $(H_w)$  Height of water above pipe, measured from pipe crown is the depth of the groundwater to the crown of the pipe. Units are in mm. Groundwater can apply additional pressure on the outer pipe wall. Groundwater depth in the form of pressure is used for pipe failure analysis and lined pipe analysis.  $0 \le H_w \le 10000 \text{ mm.}$
- Rotation angle ( $\theta$ ) Rotation angle that the lined pipe is likely to rotate to if a broken back failure occurs in the host pipe. Units are in degrees (°). Used in determining the gap spanning under ring fracture or damaged failed joints under internal pressure.  $0 \le \theta \le 5$  °.
- **Temperature change (** $\Delta T$ **)** The temperature change, fluctuations or maximum range of temperature expected to occur in the host pipe and liner during service. The difference between the maximum and minimum temperature. Units are in °C. Used to calculate any thermal effects due to, such as expansion due to temperature increase.  $0 \leq \Delta T \leq 30$  °C.
- Live load (pressure) at the burial depth ( $w_q$ ) Surface live load at the pipe burial depth based on calculations found in AS 2566.1 (1998). Live loads are used for host pipe remaining life calculations and liner life extension calculations. Units are in MPa.  $0 \le P_c \le 0.1$  MPa.

### 1.3 Step III – Host pipe defects, interface properties and others

Step III involves the host pipe defects such as pipe ovality, initial gap with and gap width due to pulling effect and hole size. Interface properties include the friction coefficient and enhancement factor. Also included is the percentage of liner service life when out of service. Further details on how each of these parameters are used can be found in the theory manual (TM M4 Part 2 – Lined pipe analysis). Figure 13 shows the user interface for Step III.

Ste	p III: Host pipe defects,	interface properties & others
	r	
Friction coefficient:	f 0.3	Gap formed due to axial movement or pulling ugp 2
Enhancement factor:	К 7 ?	Initial hole (defect) diameter (mm): d 0.00
Host pipe ovality (%):	⊿ _ 2 ?	Percentage of liner service life when out of β 0.08 ?
Initial gap width for host pipe (mm):	<i>u</i> <sub>g</sub> 10 ?	
	<b>●</b> Back	Next 🕨

Figure 13. Host pipe defects, interface properties and others

• Friction coefficient (*f*) - The friction between the liner and the host pipe (or CML). The friction coefficient ranges between 0 to 0.577 and depends on the adhesion of the liner to the host pipe (or CML). Units are dimensionless. The recommended ranges of friction coefficients for the interfaces between host pipes and polymeric liners are as follows:





Host pipe material	Friction coefficient (f)
AC or CML	0.1–0.2
Metallic	0.3–0.4

- Enhancement factor (K) Enhancement factor of the soil and existing pipe adjacent to the new pipe; a minimum value of 7.0 is recommended where there is full support of the existing pipe (dimensionless) (AWWA 2019).
- Host pipe ovality (Δ) Ovality is generally not a consideration for pressure pipe designs. However, when a pipe taken out of service is subjected to external pressures, ovality may be relevant. In absence of physical measurements, reasonable assumptions of ovality should be made based on host pipe material properties and in situ conditions. For example, ovality may never be observed on a rigid host pipe over the course of its design life, while a flexible pressure pipe may or may not ovalize (or deflect) when taken out of service (AWWA 2019). Units are percentage (%).
- **Ovality reduction factor (***C***)** Ovality reduction factor is a reduction factor based on the liner ovality. The units are dimensionless.
- Fraction of liner service life when out of service (β) The fraction of time a liner will be out of service. Used in determining if buckling of the liner may occur. Unitless. Typical values would range from 0.00005 (one day) to 0.0016 (one month).
- Initial gap width of host pipe  $(u_g)$  Initial width of a gap in the host pipe, such as a gap at a joint, broken back failure, or where there is no host pipe, that a liner can span. Units are in mm.
- Gap formed due to axial movement or pulling force  $(u_{gp})$  Width of the gap formed in the host pipe, such as a gap at a joint, broken back failure, or where there is no host pipe when the host pipe is pulled along the longitudinal axis. Units are in mm.
- Initial hole (defect) diameter (d) Initial hole or defect size (treat graphitised material as a defect), idealised to be circular in a metallic pipe. The units are in mm. Used in calculating hole spanning capability in a Class B to C liner.
- Fraction of liner service life when out of service (β) The fraction of time a liner will be out of service. Used in determining if buckling of the liner may occur. Unitless. Typical values would range from 0.00005 (one day) to 0.0016 (one month).

## 1.4 Step IV – Outputs

In Step IV, the Platform uses all the data previously filled into the first three steps to calculate either the minimum liner thickness (mm) or the service life (years), depending on the initial selection in Step I. The calculations are performed using the equations for each of the seven (or 8 if AC host pipe) limit states (see TM M4 Part 2 – Lined pipe analysis), depending on the initial Class of liner selected. The results are shown in Figure 14. The user is able to input the energy grade line slope/head loss per unit length to determine the velocity and flow rate changes from installing a liner. The Hazen Williams equation is used (TM M4 Part 2 – Lined pipe analysis).





Minimum liner	thickness T <sub>L</sub> (mm)	Dimensio	on Ratio <i>DR</i>
	90 ernal and External pressures)	39	0.07
nter EGL slope/head loss p	Velocity (for lined and unlined ca er unit length: 5 2 ?	]	td Pipe

Figure 14. Outputs from the lined pipe analysis when designing for liner thickness

## 2 NPV COST ANALYSIS

The net present value (NPV) cost analysis sub module compares three options and computes the NPV cost over a period years of analysis. It uses information from the previous modules (pipe length – from pipe library and liner lifetime – from long-term analysis) and information to be entered by users.

The three options include:

- Replace
- Rehabilitate with liner
- Do nothing

Pipe parameters needed for calculations are: length of pipe,  $L_p$  (m), years of analysis (yrs), discount rate, *i* (%) and inflation rate, *IN* (%). For each of the options the users need to input the cost per meter (rates should be either found from water utilities or contractors), the pipe or liner lifetime, and the miscellaneous/maintenance cost (annual). The do-nothing option only requires a repair/maintenance cost per failure.

The break rate of the pipe can be manually inputted by the estimated number of failures per year/100 km. Alternatively, a file can be imported that includes the year and number of breaks per 100 km, which can be gathered from risk modelling or data-driven pipe packages.

Figure 15 shows an example of the NPV analysis for the 3 options. A graph of NPV vs. the year after installation is also given. Simple results of the NPV after the years of analysis is also given at the bottom.





Pipe Parameters	NPV Graph
Pipe length:       m       80       ?         Years of analysis:       yrs       100       ?         Discount rate:       %       5       ?         Inflation rate:       %       0       ?         Option A: Replace       %m       1100       ?         New replacement lifetime:       yrs       50       ?         Miscellaneous replace cost (annual):       \$/y       0       ?         Option B: Rehabilitate with Liner       50       ?       ?         Miscellaneous liner cost (annual):       \$/y       0       ?         Option C: Do Nothing       600       ?       ?         Miscellaneous liner cost per failure:       \$       2500       ?         Select break rate:       Manual Input       Import File         Est. number of failures per year / 100 km:       2000       ?         Manual Input       Import File       ?	S50000
Results of	Net Present Value
Replace - NPV L	iner - NPV Do Nothing - NPV
- \$21344 \$1	8656 - \$79392

Figure 15. NPV analysis screen with parameters and graph

- Length of pipe  $(L_p)$  The length of the pipeline or pipe length. Units are in m.
- Years of analysis The amount of years to examine the NPV calculations for.
- **Discount rate (i)** The interest rate used to determine the present value of the liner/replacement option. Units are in %.
- Inflation rate (IN) The rate at which the prices increase over time, resulting in the lower purchasing value of money. Units are in %.
- **Cost of replace option** (*R*<sub>cost</sub>) The cost to replace the pipe using a traditional replacement method. Units are in \$/m.
- **New replacement lifetime -** The expected life of a new replacement pipe asset, e.g. 50 years. Units are in years.





- **Miscellaneous replace cost** (*R*<sub>mis</sub>) Miscellaneous costs involved for a pipe replacement option, e.g. maintenance, failures, leak repairs, etc. Units are \$/year.
- **Cost of the liner** (*L*<sub>cost</sub>) The initial cost to rehabilitate the pipe using a liner method such as CIPP or spray. Units are in \$/m.
- Liner lifetime The service life of the liner, e.g. 50 years. Calculated from long-term analysis using information from the lined pipe composite. Units are in years.
- **Miscellaneous liner cost** (*L*<sub>mis</sub>) Miscellaneous costs involved for a liner rehabilitation option, e.g. maintenance, failures, leak repairs, etc. Units are \$/year.
- **Repair/maintenance cost per failure** The average repair/maintenance cost per failure in the pipe using the do nothing approach. Units are in \$.
- Break information (break rate) Data analysis can be used by water utilities to import a file that includes the year and number of breaks per 100 km (can be gathered from risk modelling or data-driven pipe packages).
- Estimated number of failures per year / 100 km The estimated number of failures per year/100 km. This value can be gathered from Water utilities data analysis. Units are in failures/y/100 km.





# NOMENCLATURE

Liner Class (D, C, B, or A)		
Liner type (CIPP or polymeric spray)		
Reinforcement type (Glass or polymeric fibres) (for CIPP only)		
Α	Area of flow (mm <sup>2</sup> )	
С	Patch depth (mm)	
C <sub>ACe</sub>	External deterioration rate for AC pipes (mm/y)	
C <sub>ACi</sub>	Internal deterioration rate for AC pipes (mm/y)	
C <sub>d</sub>	Discharge coefficient	
$c_f$	Fatigue constant for host pipe under cyclic surge pressure	
c <sub>l</sub>	Coefficient for strength reduction	
C <sub>lc</sub>	Coefficient for creep modulus reduction	
C <sub>lf</sub>	Coefficient for fatigue strength reduction	
Cs	Intercept parameter for long-term corrosion of metallic pipes (mm)	
С	Compression modulus (GPa)	
$C_{\rm nothing}$	Cost of do nothing option (\$)	
$C_{HW}$	Hazen Williams roughness coefficient	
$C_n$	Total cash flow for each year (\$)	
$C_n(t)$	Nominal cash flow (\$) at time t	
$C_r(t)$	Real cash flow (\$) at time t	
CRF	Creep retention factor of the liner	
d	Initial hole (defect) size (mm)	
$d_f$	Future hole (defect) size (mm)	
D	Pipe internal diameter (mm)	
$D_0$	Pipe external diameter (mm)	
$D_L$	Liner external diameter (mm)	
$D_{Li}$	Liner internal diameter (mm)	
$D_M$	Mean diameter of the host pipe (mm)	
DN	Pipe nominal diameter (mm)	
$E_A$	Short-term tensile or compressive modulus of the liner in the axial direction (GPa)	
$E_L$	Short-term modulus of elasticity of the liner (GPa)	
$E_a$	Young's modulus of the adhesive (GPa)	
$E_{fh}$	Short-term flexural modulus of elasticity (hoop) of the liner (GPa)	
$E_{fhl}$	Flexural creep modulus (hoop) of the liner (GPa)	
$E_{fa}$	Short-term flexural modulus of elasticity (axial) of the liner (GPa)	
$E_{fal}$	Flexural creep modulus (axial) of the liner (GPa)	
E <sub>l,dry</sub>	Dry creep modulus of the liner (GPa)	

 $E_{l,wet}$  Wet creep modulus of the liner (GPa)



 $E_p$ 

 $E_s$ 

 $E_t$ 

 $E_{th}$  $E_{thl}$ 

 $E_{ta}$ 

 $E_{tal}$  $E_{tl}$ 

f

g

h Η

 $H_w$ 

i

IN

 $I_o$ 

k

 $k_1$  $k_2$ 

Κ

 $K_{IC}$ 

 $L_{\text{cost}}$ 

L<sub>mis</sub>  $L_c$ 

 $L_p$ 

 $L_{ps}$ 

 $n_f$ 

 $n_{PC}$ 

Ν

Ni

Р

 $P_G$ 

 $P_{GC}$ PN

 $P_N$  $P_T$ 

L



Modulus of elasticity of host pipe material (GPa) Soil modulus (MPa) Short-term tensile modulus of elasticity of the liner (GPa) Short-term tensile modulus of elasticity (hoop) of the liner (GPa) Tensile creep modulus (hoop) of the liner (GPa) Short-term tensile modulus of elasticity (axial) of the liner (GPa) Tensile creep modulus (axial) of the liner (GPa) Tensile creep modulus of the liner (GPa) Friction coefficient of the interface of the host pipe and liner Acceleration due to gravity (m/s<sup>2</sup>) Pressure head (m) Burial depth (mm) Groundwater depth (mm) Discount rate (%) Inflation rate (%) Initial investment (\$) Lateral earth pressure coefficient Patch factor Aspect ratio Enhancement factor Fracture toughness of host pipe material (MPa m<sup>1/2</sup>) Installation length of the liner (m) Cost of the liner (\$/m) Miscellaneous liner cost (\$) Critical crack length (mm) Length of the pipe (m) Length of the pipe spool (m) Cyclic surge factor Number of recurring cyclic surge pressure cycles per day Total number of surge pressure cycles for the service life of pipe/lined pipe  $n_{TPC}$ Safety factor for host pipe Factor of safety for liner imperfections NPV Net present value (\$) Operating pressure (MPa) Groundwater load (MPa) Groundwater load capacity (MPa) Nominal pressure (bar) External pressure on the liner (MPa) Test pressure (MPa)





$P_c$	Recurring cyclic surge pressure (MPa)
$P_{max}$	Maximum allowable pressure (MPa)
$P_{min}$	Minimum internal pressure (MPa)
$P_s$	Surge pressure (MPa)
$P_{v}$	Vacuum pressure (MPa)
q	Host pipe ovality (%)
$q_t$	Total external pressure on pipes (MPa)
$q_{tc}$	Liner capacity for total external pressure (MPa)
Q	Leak rate (L/s)
$r_s$	Minimum corrosion rate (long-term) of metallic pipes (mm/y)
$r_{sh}$	Lateral corrosion rate for metallic pipes (mm/y)
$r_{sv}$	Radial corrosion rate for metallic pipes (mm/y)
R <sub>cost</sub>	Cost of replace option (\$/m)
$R_h$	Hydraulic radius (m)
$R_{\rm mis}$	Miscellaneous replace cost (\$)
$R_W$	Water buoyancy factor (unitless)
S	Slope of the energy grade line, or head loss per unit length of pipe (m/m)
t	Time (years)
$t_h$	Time (hours)
Т	Pipe wall thickness allowing for uniform corrosion (mm)
$T_L$	Liner thickness (mm)
$T_{ext}$	Estimated external uniform corrosion (mm)
$T_f$	AC pipe remaining wall thickness at failure (mm)
$T_{int}$	Estimated internal uniform corrosion (mm)
$T_n$	Pipe nominal wall thickness (mm)
$u_g$	Existing gap width of host pipe (mm)
$u_{gp}$	Gap formed due to axial movement or pulling force (mm)
V	Flow velocity (m/s)
W	Traffic load (kN)
$W_s$	Live load (MPa)
$x_l$	Coefficient for strength reduction
$x_{lc}$	Coefficient for creep modulus reduction
$x_{lf}$	Coefficient for fatigue strength reduction
$y_f$	Predicted year for failure of an AC pipe (mm)
α	Coefficient of thermal expansion (mm/mm/°C)
β	Fraction of liner service life when out of service
$\gamma_s$	Soil unit weight (kN/m <sup>3</sup> )
$\gamma_w$	Unit weight of water (kN/m <sup>3</sup> )
$\Delta T$	Temperature change (°C)

 $\Delta T$  Temperature change (°C)





- $\theta$  Rotation angle (°)
- $v_L$  Poisson's ratio of the liner
- $v_p$  Poisson's ratio of host pipe material
- $\sigma_A$  Short-term tensile or compressive strength of the liner in the axial direction (GPa)
- $\sigma_{ad}$  Adhesion strength of the liner to host pipe substrate (MPa)
- $\sigma_{fh}$  Short-term flexural strength (hoop) of the liner (MPa)
- $\sigma_{fhl}$  Long-term flexural strength (hoop) of the liner (MPa)
- $\sigma_{fa}$  Short-term flexural strength (axial) of the liner (MPa)
- $\sigma_{fal}$  Long-term flexural strength (axial) of the liner (MPa)
- $\sigma_{max}$  Maximum stress in the liner (MPa)
- $\sigma_p$  Tensile stress in the host pipe (for AC pipe) (MPa)
- $\sigma_{t,AC}$  Ultimate tensile strength of AC (MPa)
- $\sigma_t$  Tensile strength of the liner (MPa)
- $\sigma_t$  Ultimate tensile strength of host pipe material (MPa)
- $\sigma_{th}$  Short-term tensile strength (hoop) of the liner (MPa)
- $\sigma_{thl,r}$  Tensile rupture strength (hoop) of the liner (MPa)

 $\sigma_{thl}$  Long-term strength (hoop) of the liner and is the lesser value of either: the tensile rupture strength (hoop),  $\sigma_{thl,r}$  (MPa) or fatigue strength (hoop),  $\sigma_{thl.f}$  (MPa)

- $\sigma_{thl.f}$  Fatigue strength (hoop) of the liner (MPa)
- $\sigma_{ta}$  Short-term tensile strength (axial) of the liner (MPa)
- $\sigma_{tal,r}$  Tensile rupture strength (axial) of the liner (MPa)
- $\sigma_{\gamma}$  Yield strength of steel (MPa)
- $\tau$  Transition period between short-term and long-term corrosion (y)
- $\Phi$  Soil friction angle (°)
- $\phi_c$  Wet creep reduction factor
- $\phi_s$  Wet strength reduction factor





# DISCLAIMER

1. Use of the information and data contained within the Lined Pipe Analysis Module is at your sole risk.

2. If you rely on the information in the Lined Pipe Analysis Module, then you are responsible for ensuring by independent verification of its accuracy, currency, or completeness.

3. The information and data in the Lined Pipe Analysis Module is subject to change without notice.

4. The Lined Pipe Analysis Module developers may revise this disclaimer at any time by updating the Lined Pipe Analysis Module.

5. Monash University and the developers accept no liability however arising for any loss resulting from the use of the Lined Pipe Analysis Module and any information and data.

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